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Structural Analysis of Fundamentally Projection Bridges Using EPS Geofoam as Backfill: A Review

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ABSTRACT: Fundamentally speaking, projection bridges are much superior than traditional projection bridges in a variety of respects. This suggests that there are fewer or none at all bridge joints, as well as less overall orientation. Both of these helps save money on the expenses of creation and maintenance. Additionally, a structure that has fewer headings and joints will need less support for the whole of its existence. Because of these reasons, several organizations are working hard to make use of basically projection bridges whenever they have the chance to. Because of this, more necessary projection bridges are being constructed in medium seismic zones for the purpose of seismic stacking. Bridges that have projections have a tendency to be more rigid than bridges that have regular projections. This is due to the fact that the superstructure is attached to the projection in a solid manner. When designing for seismic stacking, this greater inflexibility may cause the structure to attract more strengths. This may prevent the use of larger regions, which results in more expenditures in terms of both labor and materials. It would be wonderful to have the ability to create required projection bridges that are not as hardened, which would, in turn, lower the load that seismic shocks impose on specific components. Using EPS Geofoam as backfill with basic projection bridges may have a variety of advantages, one of which is that it can make the bridge less solid. It is possible that this will help reduce the strengths on the projections during seismic earthquakes and will make the bridge more adaptive as a whole.

KEYWORDS: Integral abutment bridges (IABs), Projection of Bridges, EPS Geofoam as backfill

I. INTRODUCTION

Integral abutment bridges (IABs), which may have as many as 13,000 in operation in 2005, have become more and more popular recently, according to a Federal Highway Administration survey. (P.E. Maruri and P.E. Petro) Given the numerous benefits integral proximate bridges have over derivational bridges—advantages that will be covered in more detail in this division—this growing trend makes sense. Despite a recent rise in the number of IAB construction projects, little information exists concerning their seismic presentation. The goal of this research is to better understand the seismic presentation of integrated proximate bridges with compacted versus expanded polystyrene geofoam base material in various configurations. To completely comprehend integral abutment bridge behaviour, it is necessary to comprehend how traditional bridges and integral abutment bridges differ from one another. Conventional or jointed bridges, as depicted in Figure 1, feature at least one flexibility and versatility and bearings at either end to provide little vibration in the superstructure and substructure. Rather than wheels, there are bearings. In this instance, the superstructure and the substructure are interconnected. Figure 2 depicts integrated abutment bridges, which differ from conventional bridges in that they lack expansion joints and bearings at their ends. As opposed to that, the abutments and girders are built together, or smoothly.

II. REVIEW OF LITERATURE

Extended polystyrene geofoam is frequently utilized as backfill for basic components that would advantage from less push. Since EPS geofoam is light, it can offer assistance help the stack on the soil and buildings adjacent (Geofoam Applications and Employments). EPS geofoam isn't influenced by the freeze-thaw cycle, water, or street salts. This makes it a extraordinary choice for projection backfill fabric. EPS geofoam acts like a direct versatile fabric up to a strain of almost 1%. So, to keep the geofoam from getting changeless strains that aren't great, plan loads are ordinarily restricted to the compressive resistance at 1% strain (Extended Polystyrene Information Sheet). Since EPS geofoam could be a filler that can be compressed, it isn't exceptionally firm. Since of this, it'll be simpler to compress than other materials, bringing down the load through the conventional soil mechanics handle of shear quality assembly (Handy).

EPS geofoam is additionally simple to work with and makes a difference keep construction times short since it can be put down rapidly. Indeed, in spite of the fact that the taken a toll of the materials could be a small higher than for conventional backfill, these two things offer assistance keep development costs down.

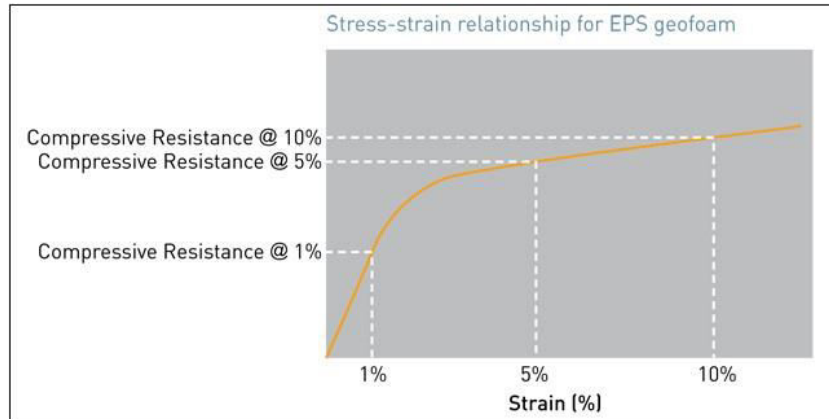


Figure1:EPS and Stress-Strain Relationship



Figure2:Geofoam Utilized as Projection Backfill EPS

SEISMIC Foundation

Modern Shirt has four primary geographical districts. the Valley and Edge Area, the Good countries Area, the Piedmont Territory, and the Coastal Plain (Witte and Monterverde). The collapsed layers of limestone, shale, and sandstone within the Valley and Edge Area make long parallel edges and wide valleys. To the east, the Good countries Area is made up of stone, gneiss, and a few marbles and has circular edges that halt and begin and profound but limit valleys (Kratzer Natural Administrations) (Witte and Monterverde). The Piedmont Area is made up of sandstone, basalt, shale, and diabase. It could be a rolling plain with a number of higher edges. It is isolated from the Good countries Territory by the Ramapo Blame and other major deficiencies. Edges and uplands within the Piedmont Area, It is level and doesn't have any slopes or uplands. Since seismic tremors can happen nearly anyplace and have wide-ranging impacts, each bridge, no matter where it's found, is at chance of being harmed by a harming seismic tremor. Since the 1700s, when early verifiable records got to be accessible, there have been a few seismic tremors in Unused Shirt. Numerous of these seismic tremors were felt in a expansive portion of the state and, in a few cases, in other states as well.

In expansion to the seismic tremors that happened in Unused Shirt, there were a few others that happened somewhere else but may be felt within the state. In 1939, a seismic tremor hit close Salem District, Modern Shirt, and individuals in Baltimore, Maryland, and Philadelphia, Pennsylvania, seem feel it. In 1973, a greatness Moreover, the harm and impacts of a seismic tremor can be felt for miles. It's not unordinary to go hundreds of miles. Since of this, all bridges, no matter where they are, may be harmed by seismic tremors and ought to be made in a way that takes this into consideration.



Geotechnical Information

All of the information about the soil that could be found was collected, because it would be needed to make soil profiles and soil springs for each bridge. The data was put together by looking at existing bridge plans, both contract and as-built plans, as well as past soil boring programmes, evaluation reports, and geotechnical considers. After looking at the soil data that was already available, it was decided that each of these bridges would need more borings to get enough data to start making soil profiles for each foundation at each bridge location. Soil data from previous walks was available for Bridge A, and two more SPT borings, one at each projection, were also done. The main bore was in the right place north of the south projection, and the small bore was in the right place south of the north projection. After a layer of fill that was between 10 and 15 feet thick, there was a layer of free sand that was 35 feet thick, according to the soil profile that was made from these modern borings and the data from the old borings. Under the free sand, there could be a layer of sand 20 feet thick and then a layer of sand that is very thick.

Between the south projection and Dock 1, the boring was as near to the south projection because it may well be. The soil profile was made by putting together the information from the unused SPT borings and the information from the past borings. The soil-profile appears the 15 to 20 feet deep and is made up of medium-dense sand. Underneath it may be a layer of thick to medium-dense sand. After the sand layer, there are 10–45-foot-thick pockets of solid clay and peat. The final layer is made of exceptionally thick sand.

III. CONCLUSION

Based on the examination of the 13 models per bridge, 1 conventional joint bridge with seat-type projections and 12 with necessarily projections, as well as the distinctive sorts of backfill and soil around the bridge, a few conclusions can be drawn. The most conclusions are the taking after.

- The basic period is much shorter for bridges with indispensably projection than for bridges with customary projections. This implies that bridges with necessarily projection are less adaptable than bridges with normal abutments.
- The length of the bridge may influence how much more the projection moves when utilizing EPS geofoam rather than compacted backfill.
- Changes in wharf balance minutes and shears may depend on the length of the bridge. When EPS Geofoam backfill is utilized rather than compacted backfill, the sum of alter in pier footing minutes and shears is less the longer the bridge is. It isn't clear in case this is often a coordinate connect or on the off chance that this alter is caused by something else.

REFERENCES

1. American Institute of Steel Construction (AISC). *Manual of Steel Construction*, 14th Edition. Chicago, IL, 2011.
2. Albhaisi, Suhail M. and Hani Nassif. "Effect of Substructure Stiffness on the Performance of Integral Abutment Bridges Under Thermal Loads." (2012). 325.
3. Indian Association of State Highway Transportation Officials. *BRIDGE LRFD Bridge Design Specifications*, Sixth Edition. Washington, D.C., 2012.
4. Bowles, J.E. *Foundation Analysis and Design*, 5th Edition. New York, NY. McGraw-Hill, 1996. —. *IAB 2015 Advanced w/Rating Version 17.1.1 Build 1099*. 2014.
5. Dicleli, Murat, P. Eng and M. Suhail Albhaisi. "Maximum Length of Integral Bridges Supported on Steel H-Piles Driven in Sand." *Elsevier* (2003). 14.
6. Clough, G. W. and J. M. Duncan. "Earth Pressures." Fang, Hsai-Yang. *Foundation Engineering Handbook*. Springer, 1991. 923.
7. Computers & Structures, Inc. *CSI Analysis Reference Manual for SAP2000, ETABS, SAFE and IAB*. Berkeley, California, 2014.
8. Dombroski, Jr., Daniel R. "Earthquake Risk in New Jersey." 2005.
9. *Expanded Polystyrene Data Sheet*. 2015. 15 August 2015. <<http://universalconstructionfoam.com/resources/expanded-polystyrene-data-sheet.php>>.
10. Frosch, J. Robert, et al. "Jointless and Smoother Bridges. Behavior and Design of Piles." Perdue University, 2004.
11. *Geofoam Applications and Uses*. 2015. 2015. <<http://www.geofoam.com/applications/>>. Insulated Building Systems. "GeoTech TerraFlex Product Specifications." n.d.
12. "Integral Abutment and Jointless Bridges." *The 2005 - FHWA Conference*. Baltimore, MD. Federal Highway Administration, 2005. 343.



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